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Coda-wave monitoring of continuously evolving material properties and the precursory detection of yielding

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The nominally incoherent coda of a scattered wavefield has been shown to be a remarkably sensitive quantitative monitoring tool. Its success is, however, often limited to applications where only moderate or localised changes in the scattered properties of the medium can be assumed. However, the compressional deformation of a relatively homogeneous rock matrix towards failure represents a monitoring wavefield pronounced changes in both velocity and scattering power often due to a distribution of inelastic changes. A rolling reference wavefield is implemented when applying coda-wave interferometry (CWI) and coda-wave decorrelation (CWD), allowing relative velocity and material scattering power monitoring for such applications. It is demonstrated how this modification enables the qualitative monitoring of stages in material deformation common to unconfined compressive strength tests. In addition, the precursory/subtle onset of material yielding is identifiable in both the CWI and CWD methods, which was not possible when comparing to a fixed reference wavefield. It is, therefore, expected that this approach will enable these coda-based methods to robustly monitor continuous, destructive processes at a variety of scales. Possible applications include critical infrastructure, landslide, and reservoir compaction monitoring where both the subtle continuous and sudden large changes in a material’s scattering properties occur.


[RDC] Pages: 1060–1068

I. INTRODUCTION

With the advent of affordable sensor networks, along with the ability to transmit large amounts of data wirelessly, the feasibility of evermore complex monitoring systems is increasing rapidly (Brownjohn, 2007; Stähli et al., 2015). Whether it be critical infrastructure structural health monitoring (bridges, dams, nuclear reactors) or environmental hazards (earthquakes, landslides, induced seismicity) or early warning detection to dynamic failure, continuous non-intrusive monitoring is crucial.

Many works have demonstrated the remarkable sensitivity of the scattered wave to media-wide properties, through the estimation of velocity changes over fault zones (Poupinet et al., 1984), in volcanoes (Grêt, 2005; Matsumoto et al., 2001; Radomopurbo and Poupinet, 1995; Sens-Schönfelder and Wegler, 2006), landslides (Mainsant et al., 2012), and within the lunar near surface (Sens-Schönfelder and Larose, 2008). Various ultrasonic laboratory experiments have shown the coda’s sensitivity to changes at the mesoscopic scale in terms of stress, temperature, and water saturation (Grêt et al., 2006; Stähler et al., 2011; Zhang et al., 2012). The coda-wave interferometry (CWI) formulation presented by Snieder et al. (2002) has served as the basis for many such relative time-lapse monitoring studies on solid media. In the field of optics diffusing wave spectroscopy (DWS) (Pine et al., 1990) has been used to study different aspects of strongly scattered light, and was later extended to acoustics as diffusing acoustic wave spectroscopy (DAWS) (Page et al., 2000) relating phase changes in a wavefield to, for example, monitor the average displacement of scatterers in a fluidised suspension.

In general, the recent research applying CWI has focused on exploiting the sensitivity benefits of the coda-wave in monitoring subtle, often cyclical, non-destructive processes and thereby inferring the average velocity change. To the authors knowledge, the only studies which have applied CWI well into a region of inelastic deformation have been limited to concrete structures either partially (Zhang et al., 2016) or fully under tensile forces (Zhang et al., 2012), of which only the latter shows the ability to track velocity changes close to the ultimate strength of the structure.

The method termed coda-wave decorrelation (CWD) as introduced by Larose et al. (2010) and later formalised by Rossetto et al. (2011) provides a spatiotemporal theoretical expression for the resulting decorrelation between a reference and perturbed wavefield due to the addition of a single or multiple localised scatterers (Planès et al., 2015). The ability of CWD to locate such a local change in scattering properties with the aid of a maximum likelihood inversion between the measured and theoretical decorrelation has been shown in a laboratory setting (Larose et al., 2015, 2010), around an active volcano (Obermann et al., 2013a) and most recently on a life-sized reinforced concrete structure (Zhang et al., 2016).

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II. FIXED-REFERENCE MONITORING OF DESTRUCTIVE PROCESSES

A. The theory of CWI for monitoring changes in velocity

The formulation of CWI as presented by Snieder et al. (2002) rests upon the understanding that a recorded waveform $u_p(t)$ which has interacted with an unperturbed scattering medium can be represented as the summation of all possible paths $P$ through that medium as

$$ u_p(t) = \sum_P A_P(t), $$

where $A_P$ denotes the wave that propagates along path $P$.

The first major assumption concerning the medium itself is that each individual scatterer has stationary properties, therefore, preventing a change in its scattering cross-section (size, shape, density, and velocity). Additionally, it is assumed that the mean free path $l$, which is indirectly related to the averaged distance between scatterers is much greater than the dominant wavelength $\lambda$.

A perturbed waveform $u_p(t)$ that has experienced a subtle change $\delta \ll l$ in either the location of scatterers, the location of the source, or the background medium velocity, can then be represented as

$$ u_p(t) = \sum_P A_P(t - \tau_P), $$

where $\tau_P$ represents the travel time change along path $P$. This formulation, therefore, implies that provided the perturbation does not change the dispersion of the wavefield; only a change in the travel time of $u_p(t)$ will occur. The cross-correlation coefficient $CC(t_c)$ for a particular window of width $2\tau_c$ centered at time $t_c$ within the coda, $(t_k - \tau_c)$ to $(t_k + \tau_c)$ is often presented as

$$ CC(t_c) = \left[ \frac{\int_{t_k - \tau_c}^{t_k + \tau_c} u_p(t)u_p(t + t_c)dt}{\int_{t_k - \tau_c}^{t_k + \tau_c} u_p^2(t)dt} \right]^{1/2}, $$

where $u_p(t)$ and $u_p^2(t)$ are the unperturbed and perturbed waveforms, respectively. This time shifted cross-correlation coefficient $CC(t_c)$ will reach a maximum when the average travel time perturbation $\langle \tau \rangle$ across all perturbed paths $P$ is

$$ \langle \tau \rangle = t_c. $$

A homogeneous relative velocity perturbation can then be determined to a first order approximation by

$$ \frac{\partial v}{v} = -\frac{\langle \tau \rangle}{t_c}. $$

If we now consider this in terms of a rock matrix, the first assumption of CWI implies that the size, width, and number of inhomogeneities (e.g., fractures, pores) remains constant in time. Furthermore, it requires that for a change in the phase to dominate the average distance between such inhomogeneities (the mean free path) is considerably larger than a wavelength. The constraint of a non-dispersive perturbation requires that changes in properties such as scattering or intrinsic attenuation are negligible. Provided only a subtle, cyclical, elastic deformation is applied to a medium with a sufficiently high source frequency, these assumptions generally hold within the time period between the perturbed and unperturbed wavefield. This is demonstrated for a rock matrix in the work by Grêt et al. (2006), where CWI derived
velocity changes are observed in a sandstone core sample due to a subtle elastic increase in uniaxial compressive stress.

B. Applying a CWD approach to monitoring changes in scattering properties

In order to enable the coda-based monitoring of scenarios where inelastic material deformation occurs, one can focus on the maximum cross-correlation coefficient \( C(t_s) \) instead of the \( t_s = \langle t \rangle \) at which its maximum is found. This is equivalent to the CWD methods discussed earlier, though with the distinction that here the main goal is not to find a single (Larose et al., 2010) or several localised scatterer perturbations (Planès et al., 2015) within the medium but to monitor a global perturbation within the region sampled by the coda-wave.

The scattering power of a medium can be defined in terms of its total scattering coefficient \( g_0 \) (Aki and Chouet, 1975). Assuming an idealized scattering media as a random distribution of \( n \) point-like scatterers with number density \( \rho_n \), within a background velocity \( V_0 \), and a propagating plane wave this coefficient can be defined as

\[
g_0 = \rho_n \sigma_T \equiv l^{-1}, \tag{6}
\]

which is inversely proportional to a material’s mean free path \( l \). By applying diffusive propagation theory, Rossetto et al. (2011) derived an expression for the theoretical decorrelation caused by localised perturbation of the total scattering cross-section \( \sigma_T \). This requires the knowledge of a sensitivity kernel \( Q(s, s_m, r, t) \) between a source \( s \) and receiver \( r \) for the perturbation location \( s_m \) and a time \( t \) within the coda. Re-writing this in terms of a change in a material’s total scattering coefficient \( g_0 \) with a background velocity \( V_0 \), explicitly in terms of unperturbed and perturbed medium states gives

\[
K^T(X, t) = \frac{V_0}{2} \int_{V_0} |\Delta g_0| \cdot Q(s, x, r, t) dV_Q(x), \tag{7}
\]

where \( X \) defines the ensemble of \( n \) randomly distributed perturbation locations within the kernel volume \( V_Q \), while \( |\Delta g_0| \) locally defines the difference in total scattering coefficient between the perturbed and unperturbed material states. Considering Eq. (7) in terms of the evolution of a material’s scattering properties, any change in the size or impedance contrast of the scatterers will result in an increase in the decorrelation, as will the addition or removal of scatterers as the number density \( \rho_n \) changes. Furthermore, these changes will alter the intensity of the diffuse wavefield (Paaschens, 1997; Pacheco, 2015) through the material’s diffusion and attenuation coefficients, resulting in a change in the sensitivity kernel.

Equation (7) is, therefore, able to describe both a change in the number of scatterers, through \( \rho_n \) or in the size and impedance contrast of scatterers, through \( \sigma_T \), provided one can assume the same sensitivity kernel \( Q(s, x, r, t) \) in both states. For a large perturbation in \( g_0 \), it will become difficult to maintain this approximation as the diffusion and attenuation coefficients between the two medium states begin to diverge. In terms of the measured data, the decorrelation coefficient \( K(t_s) = 1 - CC(t_s) \) between two recorded time-series will approach one, at which point it will only represent a spurious correlation between two time-series. However, provided that a weak perturbation can be ensured in the time interval \( t_u - t_p \), the measured \( K(t_s) \) can be related to the modulus of the change in a material’s scattering coefficient weighted by the sensitivity kernel.

III. ROLLING-REFERENCE CODA MONITORING OF DESTRUCTIVE PROCESSES

With the goal of ensuring the assumptions of CWI and CWD are satisfied throughout the long term monitoring of continuously evolving material scattering properties, we propose the use a rolling reference waveform when determining both the decorrelation coefficient and the relative velocity change. While the idea of manually selecting a different reference waveform during monitoring is not new (Grêt et al., 2006; Obermann et al., 2013b), the uses of a rolling reference waveform enabling the continuous monitoring of a material throughout deformation up until catastrophic failure remains unexplored. With regard to Eq. (3), such a modification requires the fixed reference waveform \( u_s(t) \) to be replaced by a rolling reference waveform \( u_p(t) \), in a monitoring sense lagging behind the most recently acquired waveform \( u_p(t) \) by \( N \) measurements. In terms of the decorrelation coefficient,

\[
K^{roll}(t_s) = 1 - \frac{\int_{t_u}^{t_u + t_u} u_{p_j,0}(t) u_p(t + t_s) \, dt}{\int_{t_p}^{t_p} u^2_{p_j,0}(t) \, dt \int_{t_u}^{t_u + t_u} u^2_p(t) \, dt}^{1/2}, \tag{8}
\]

Provided the repeat measurement period \( dt_{pj-N} \), at which each \( u_{p_j}(t) \) waveform is recorded is much smaller than the rate of change in the scattering coefficient \( dg_0/dt_{pj-N} \), this formulation provides the flexibility of selecting a sufficiently small reference lag \( N \) such that each correlation is able to better satisfy the assumptions of CWI and CWD. In terms of Eq. (7), for a small \( dt_{pj-N} \) the measured \( K^{roll} \) and the associated \( t_s \) provides a qualitative description of the rate-of-change \( (g_0 = dg_{0,p_s,j}/dt_{pj-N}) \) in \( g_0 \) and \( dv/v \), respectively, throughout the monitoring period.

IV. EXPERIMENTAL SETUP

In order to experimentally analyse and compare the attributes of fixed and rolling reference forms of CWI and CWD for continuously evolving scattering properties, acoustic monitoring of UCS tests on laboratory scale core samples is made, see Fig. 1. For redundancy, one axial and two radial mounted transducers acting as receivers-RX, and one axially mounted source-TX transducer are attached to the surface of a core sample. Transducers with a peak operating frequency of 1 or 2.25 MHz are used depending on the experiment. In order to reduce the presence of noise, the stacking of 512 individual wavelets is made, resulting in a repeat
measurement period $dt_{pq-N}$ of $15$ s. An axially increasing force $F$ is applied to each core sample controlled by a constant axial strain rate ($\Delta L/Ldt$) between $1.32-4 \times 10^{-6}$ s$^{-1}$. The applied force $F$ and total axial displacement are recorded every half second throughout monitoring up until the sample experiences dynamic failure. The lithologies tested are Bentheimer sandstone (Peksa et al., 2015), which has well sorted grain sizes (180–320 $\mu$m) and high porosity (21%–27%) and a poorly sorted (30–800 $\mu$m), zero primary porosity granite sourced from Benin. A summary of the three core sample UCS experiments is provided in Table I along with the abbreviated sample names employed in this work.

V. CWI AND CWD MONITORING OF AN UCS TEST

Monitoring the evolution of a rock matrix towards dynamic failure with a wavelet at the mesoscopic scale (Barnhoorn et al., 2010, 2018; Heap and Faulkner, 2008; Li et al., 1998), is characterised by the continuous evolution of its wavefield propagation properties. For UCS tests, this can be illustrated by considering the different stages of material deformation as shown in Fig. 2 for the BNT1 core sample. First, the initial closure of any existing fractures or pore-space occurs. This is followed by the elastic deformation where a linear stiffening of the rock matrix is expected. These first two stages represent, in terms of Eq. (6), a general reduction in the total scattering cross-section $\sigma_T$ and, therefore, $g_0$ of the medium, as the size and then impedance contrast of the fractures reduces. Finally, the onset of inelastic deformation marks the beginning of fracture growth/formation often termed the fracture initiation and growth threshold (FIGT) (Heap and Faulkner, 2008). The growth and addition of new fractures represents an irreversible increase in $\sigma_T$ and number density of fractures $\rho_n$, respectively. Considering these changes in material scattering properties over a typical UCS test, it becomes difficult to make the assumption of a phase dominated or weak change in $g_0$ necessary in order to apply either CWI or CWD for a fixed initial reference. Focusing first on the CWD method, in Fig. 2 we compare the sensitivity of a fixed $K_k^{\text{fix}}$ versus a rolling $K_k^{\text{roll}}$ reference decorrelation coefficient to the stages of material deformation common to UCS tests. The $K_k^{\text{fix}}$ coefficient shows a rapid increase over the first 0.025% of axial strain, followed by a more gradual slope towards 0.7 at the onset of elastic deformation. For the remainder of the USC test $K_k^{\text{fix}}$ shows little sensitivity to the ongoing deformation, with only a pronounced increase as dynamic failure occurs at 0.56% axial strain. This indicates initial fracture closure represents a large perturbation in $g_0$, though after this point, $K_k^{\text{fix}}$ provides little informative value other than the knowledge that some notable change occurred early on during the monitoring period. In comparison, $K_k^{\text{roll}}$, which is related to the relative rate of change in $g_0$ as calculated by Eq. (8), shows clearly identifiable trends segmenting each of the defined regions of deformation.

- The initial 0.025% of axial strain results in an increase in the rate of change of decorrelation $K_k^{\text{roll}}$. This indicates the initial deformation represents an increase in the rate of fracture closure, and, therefore, an increase in the rate of reduction in the material’s scattering coefficient (i.e., curvature in $\dot{g}_0 < 0$) is expected as the impedance contrast of inter- or transgranular fractures is reduced.
- This is followed by a gradual reduction in $K_k^{\text{roll}}$ indicating a reducing rate of fracture closure (i.e., curvature in $\dot{g}_0 > 0$) as the rock matrix stiffens leading up to the beginning of elastic deformation.
- The elastic deformation of the material is characterised by a constant, low $K_k^{\text{roll}}$ and, therefore, $\dot{g}_0$, where $\dot{g}_0 = 0$. Here a unit stiffening of the rock matrix results in a proportional change in the impedance contrast of fractures and, therefore, $g_0$.
- At the FIGT inelastic deformation occurs, which results in an increase in $\dot{g}_0$ where $\dot{g}_0$ and $\ddot{g}_0 > 0$. These changes are reflected by initially a gradual and then steep increase in $K_k^{\text{roll}}$ all the way until dynamic failure. This initial gradual increase is possibly evidence of the sub inelastic region of stable fracture growth before unstable growth continues, noted in literature for crystalline and brittle rocks (Bieniawski, 1967).

In order to make a direct comparison with $K_k^{\text{fix}}$, the scaled $(0-1)$ average cumulative summation of each correlation window’s decorrelation coefficient $(K_k^{\text{sum}})$ is also provided. A general agreement is evident between $K_k^{\text{fix}}$ and $K_k^{\text{sum}}$ as both show an initial increase, stabilisation, and then final increase within the region of elastic deformation, however the latter clearly shows an improvement in its sensitivity to the underlying perturbations. Most notably, $K_k^{\text{sum}}$ reflects the linear region of elastic deformation as the applied constant

![FIG. 1. (Color online) Schematic representation of the UCS core test setup with active acoustic monitoring. An axially mounted MHz source (TX) with one axial and two radial receives (RX) monitoring the changes in scattering properties every $\Delta t$ for increasing load $F$ and axial deformation $\Delta L$.](image)

### TABLE I. Summary of the UCS tests core sample length and diameter, lithology, strain rate, and acoustic source frequency.

<table>
<thead>
<tr>
<th>L/D (mm)</th>
<th>Lithology</th>
<th>$\Delta L/Ldt$ (s$^{-1}$)</th>
<th>Src. (MHz)</th>
</tr>
</thead>
<tbody>
<tr>
<td>BNT1</td>
<td>Bentheimer sandstone</td>
<td>$4.0 \times 10^{-6}$</td>
<td>1.0</td>
</tr>
<tr>
<td>BNT2</td>
<td>Bentheimer sandstone</td>
<td>$4.0 \times 10^{-6}$</td>
<td>1.0</td>
</tr>
<tr>
<td>GRA</td>
<td>Benin granite</td>
<td>$1.3 \times 10^{-6}$</td>
<td>2.25</td>
</tr>
</tbody>
</table>
strain rate results in a linear stiffening of the medium (i.e., a change in $\sigma_T$) and the transition to the non-linear region of elastic fracturing. The smoothed appearance of $K_{\text{sum}}$ is a result of its calculation from the average of each independent correlation windows cumulatively summed decorrelation coefficient.

In making a similar comparison for CWI monitoring of the BNT1 dataset, the differences in sensitivity between the fixed and rolling reference relative velocity changes as determined by Eq. (5) are compared, see Fig. 3. The fixed reference $\left[\frac{\Delta v}{v}\right]_{\text{fix}}$ shows an initial coherent increase in velocity inline with the time of flight (TOF) changes $\left[\frac{\Delta v}{v}\right]_{\text{TOF}}$ derived from the first arrivals, though after only the first 0.12% of axial strain its trend appears incoherent. In contrast, the rolling reference derived cumulative summation $\left[\frac{\Delta v}{v}\right]_{\text{sum}}$ curve tracks the $\left[\frac{\Delta v}{v}\right]_{\text{TOF}}$ curve plateauing over the elastic region, before decaying with increasing inelastic deformation. A maximum in $\left[\frac{\Delta v}{v}\right]_{\text{sum}}$ occurs at 22.8 MPa whereas the $\left[\frac{\Delta v}{v}\right]_{\text{TOF}}$ maximum occurs at 35 MPa. The initial increase in relative velocity is explained by acoustoelastic theory, while any subsequent reduction in velocity occurs due to the formation of the first micro-cracks (Selleck et al., 1998; Shah and Hirose, 2010). The earlier reduction in $\left[\frac{\Delta v}{v}\right]_{\text{sum}}$ indicates the expected improvement in sensitivity of CWI to these inelastic changes (Zhang et al., 2012). The presented $\left[\frac{\Delta v}{v}\right]_{\text{sum}}$ is the average relative velocity change determined from each of the nine correlation windows shown over a single waveform.

VI. PRECURSORY IDENTIFICATION OF MATERIAL YIELDING

Based on the presented ability of both $K_{\text{roll}}$ and $\left[\frac{\Delta v}{v}\right]_{\text{sum}}$ to monitor deformation of a rock matrix, we will now assess both methods sensitivity to the onset of inelastic deformation. The yield point of a material is often defined as the transition from predominantly stable fracture growth to predominantly unstable fracture growth (Bieniawski, 1967; Mogi, 2007), while others define it as the point the first micro-fractures are formed (Elliott and Brown, 1986; Paterson and Wong, 2005). There is, however, general consensus that the yield point can be identified on a stress/strain diagram as the onset of non-linear behaviour which follows in Fig. 2, and for the purpose of this study is only considered as a qualitative indicator of the material’s velocity change.

In summary when applying CWD the three stages of material deformation from initial fracture closure, elastic deformation, and fracture growth are all reflected in the $K_{\text{roll}}$ and $K_{\text{sum}}$ trend lines, whereas it is difficult to find any clear segmentation in the $K_{\text{fix}}$ curve. The application of a rolling reference CWI derived $\left[\frac{\Delta v}{v}\right]_{\text{sum}}$, while not able to clearly segment each region of deformation does track the TOF derived changes with improved sensitivity to the onset of inelastic deformation, whereas the fixed reference $\left[\frac{\Delta v}{v}\right]$ does not.
a linear elastic region. This parameter is of critical importance in the prediction of dynamic material failure as it signifies that permanent material deformation has begun. Typically, the yield point from a UCS test would be determined by hand, though in order to remove some of the ambiguity surrounding this, an automated search is made based on the method described in the Appendix. On the basis of this yield point the precursory/subtle detection capabilities of the CWD and CWI methods can be assessed.

For this purpose two additional UCS tests are made, on a repeat Bentheimer (Fig. 4—BNT2) and Benin granite (Fig. 5—GRA) core sample. As with BNT1, the indication of inelastic deformation (FIGT) is identified at the end of the trough in $K_{roll}$ as the rate of change in $g_0$ increases. Similarly, the coda and first arrival derived relative velocity change inelastic indicators are identified at the onset of a reduction in velocity as the acoustoelastic effect (Toupin and Bernstein, 1961) working to increase the velocity is overcome by the formation of the first micro-fractures (Zhang et al., 2012). In order to make a comparison between the different lithologies and datasets, each precursory indicator is quoted in terms of the percentage of the yield stress at which it is found, as presented in Fig. 6.

For the Bentheimer sandstone samples (BNT1 and 2) the CWI cumulative relative velocity changes $[\delta v/v]_{\text{sum}}$ indicate the onset of yielding at around 70% of the yield stress, where the CWD decorrelation coefficient $K_{roll}$ begins to increase at around 86% of the yield stress. The GRA sample, on the other hand, shows $K_{roll}$ as providing the earliest indication of inelastic deformation at 66% in comparison to $[\delta v/v]_{\text{sum}}$ at 89% of yield stress. This may be a result of the differences in fracturing behaviour between the well sorted quartz-rich, porous grains of the Bentheimer sandstone and the poorly sorted non-porous grains of the Benin granite resulting in reduced sensitivity of the velocity change at the onset of fracturing.

In all cases, the reference $[\delta v/v]_{\text{TOF}}$ shows no precursory indicative power to material yielding with the onset of decay occurring after the stress-strain identified yield point. The lack of sensitivity of TOF changes to the classically defined yield point is also noted in the work by Barnhoorn et al. (2018). Refer to the Appendix for a description of the method applied to determine the yield point.

VII. DISCUSSION AND CONCLUSIONS

In this study, we have shown that by applying a rolling reference waveform in both the CWI and CWD form, one is able to monitor the evolution of a material’s relative velocity change and scattering coefficient where a large inelastic perturbation in its scattering properties occurs. Furthermore, we demonstrate that both approaches provide precursory indication of material yielding as determined from stress-strain measurements. While these benefits have direct applications in terms of providing a better understanding of rock properties in the laboratory, it is expected that such a processing approach will enable a wider range of field scale monitoring applications.

In the structural concrete bending tests by Zhang et al. (2016), which applied fixed reference CWI and CWD and the work by Larose et al. (2015), where only CWD was applied the authors noted the increase in error of both relative velocity and the decorrelation coefficient as inelastic deformation progressed. While this can be easily overcome by the manual selection of a new reference point during monitoring, we suggest that where the long term, continuously monitoring of both gradual and sudden inelastic changes is required, a rolling reference is the preferred
FIG. 4. (Color online) CWD and CWI monitoring of BNT2. The $K^{\text{roll}}$ (FGIT: 22.5 MPa) and $\frac{1}{2} \sum_{\text{corr}} (\text{max: } 19.3 \text{ MPa})$ trends are generated with a rolling lag equal to the repeat measurement period of 15 s (i.e., $N = 1$), calculated from the average of the eight independent 0.04 ms correlation windows. For comparison the Time Of Flight derived $[\delta u/v]_{\text{TOF}}$ (max: 34 MPa) is provided.

FIG. 5. (Color online) CWD and CWI monitoring of GRA. The $K^{\text{roll}}$ (FGIT: 101 MPa) trend is generated with a repeat measurement period of 150 s ($N = 10$) from the average of the ten independent, 0.013 ms correlation windows shown. The $[\delta u/v]_{\text{corr}}$ (max: 138 MPa) cumulative summation is made for $N = 1$. For comparison the Time Of Flight derived $[\delta u/v]_{\text{TOF}}$ (max: 190 MPa) is provided.
choice. In terms of structural applications, this allows for the gradual long term monitoring of micro-cracking which occur throughout the life of concrete while maintaining sensitivity as an early warning to material failure. Furthermore, to the authors knowledge, all application of CWD where notable inelastic deformation occurs have been under a bending force resulting in localised micro-cracks (Paterson and Wong, 2005) where nucleation occurs within a region of tensile stress. As supported by the current study, where a homogenous compressional stress field is applied, the issue of rapid decorrelation is likely to become more immediately evident where a more homogeneously distributed fracture network forms.

An active source landslide or rock slide is an example of a near surface environmental geophysics application, where long term monitoring for the purpose of early warning detection to dynamic failure is required. Passive monitoring has been shown to be sensitive to a continuous velocity change, prior to a fluid slippage event (Mainsant et al., 2012), where a rapid reduction in rigidity within a liquefaction region was identified as the mechanism for initial dislocation. Pore-pressure driven reservoir compaction is a process, which is by definition, controlled at the grain scale (Hol et al., 2015) with the first large scale induced seismicity events often along existing faults occurring decades after the beginning of production (Bourne et al., 2014; van Thienen-Visser and Breunese, 2015). An in borehole active source continuous monitoring setup (Zoback et al., 2011), will perhaps be able to identify both the gradual inelastic intergranular compaction (Pijnenburg et al., 2018) as well as the onset of larger scale transgranular cracking which results in surface subsidence (Pijpers and Van der Laan, 2016; Pratt and Johnson, 1926; Sharp and Hill, 1995). Where long term continuous monitoring is required, it is expected that rolling reference processing will help fulfill some of the criteria for early warning systems as well as the long term monitoring of a material’s scattering properties.

In the context of this study, the ability to determine the subtle onset of material yielding was possible with only the knowledge that loading occurs at a constant strain rate, and in the absence of direct stress and strain measurements. Provided the employed source produces wavelengths sensitive to the perturbations in scattering, sensitivity to the mentioned structural-health, landslide, and compaction related monitoring is expected. While the feasibility of such applications requires additional investigation, they can all be generalised as monitoring scenarios where both high rate-of-change and long-term transient changes in material scattering properties are of concern. It is, therefore, suggested that with an approximate knowledge of the underlying direction and type of perturbation, rolling reference coda-based monitoring can be a useful addition to the mentioned early warning systems as well as the long term monitoring of a material’s scattering properties.

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APPENDIX A: YIELD POINT IDENTIFICATION

The yield point of a stress($\sigma$)-strain($\epsilon$) curve is determined by a search for the most linear region. This is achieved by fitting $q$ lines of length $m$ to the trend as described by

$$\sigma_i^{fit} = \beta_0 + \beta_1 \epsilon_i^{m,fit}, \quad (A1)$$

and assessing the most linear on the conditions that the slope $\beta_1$ is at a maximum and the fitting error at the end of the line is less than 2%.

$$\beta_{q,1} > \beta_{q-1,1}, \quad \Delta(\sigma_{q,m}^{fit}, \sigma_{q,m}) < 2\%.$$  \quad (A2)

Here $m$ is selected to be approximately half the length of the linear elastic region in terms of sample points. The yield point is then defined at the end of the fitted line with maximum slope $\beta_1$ and fitting error <2%.


